



**SIDDHARTH GROUP OF INSTITUTIONS :: PUTTUR**  
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**QUESTION BANK**

**Subject with Code: DESIGN OF ADVANCED CONCRETE STRUCTURES (18CE1017)**

**Course & Branch: M. Tech - Structural Engineering**

**Year & Sem: I M.TECH & II-Sem**

**Regulation: R18**

**UNIT-I**

**ESTIMATION OF CRACK WIDTH IN REINFORCED CONCRETE MEMBERS**

1. A simply supported rectangular beam 300 mm x 500 mm, having an effective span of 6 m, is subjected to UDL of 16 KN/m, inclusive of its self weight. The beam is reinforced with 3 bars of 20 mm diameter, at an effective cover of 50 mm. Assuming M20 concrete and Fe415 steel. Calculate the surface Crack width at the following locations. [12M]
  - At a point 'A' directly under a bar on tension face
  - At the bottom corner 'B' of the beam
  - At a point 'C' distant  $2(d-x)/3$  from N.A, where crack width is likely to be maximum.
2. a) Calculation of Crack width in Beams [6M]
  - b) Factors affecting Crack width in beams [6M]
3. A simply supported T-beam span of 5 mts is subjected to a moment of 85 KN/m at mid span. The section of beam is as shown in figure. Calculate the crack width at corner A, directly under tension reinforcement B & the center tension face C. the materials are M20 grade concrete and Fe415 steel. . [12M]
4. a) Explain moment curvature relation of reinforcement concrete sections [6M]
  - Factors affecting Crack width in beams [6M]
5. A beam of AB span 4 mts fixed at one end and freely supported at other end carrying a UDL of 30 KN/m at collapse. Draw maximum BM as per recommendation of code IS 456-2000 for redistribution of moment. . [12M]

6. (a) What are the major factors which influence the crack width in flexural members? [6M]
- a) A beam of width 450 mm, depth 700 mm cover of reinforcement 40 mm is reinforced with 3 rods of 40 mm diameter. Determine the crack width when the section is subjected to a BM of 490 Kn m at a point on the side of the beam 250 mm below the neutral axis.
7. (a) What are the major factors which influence crack-widths in flexural members? [6M]
- (b) Discuss the issues involved in designing for achieving control over thermal and shrinkage cracks in large R.C structures. [6M]
8. (a) What are the different options available to a designer with regard to control of cracking in flexural members? [6M]
- (b) Are the normal designing requirements of the code adequate for ensuring crack-width Control ? Give comment on this. [6M]
9. A simply supported rectangular beam 400 mm x 600 mm, having an effective span of 6 m, is subjected to UDL o 16 KN/m, inclusive of its self weight. The beam is reinforced with 3 bars of 20 mm diameter, at an effective cover of 40 mm. Assuming M30 concrete and Fe415 steel. Calculate the surface Crack with at the following locations. [12M]
- At a point 'A 'directly under a bar on tension face
- At the bottom corner 'B 'of the beam
- At a point 'C 'distant  $2(d-x)/3$  from N.A, where crack width is likely to be maximum
10. A beam of width 500 mm, depth 700 mm cover of reinforcement 50 mm is reinforced with 3 rods of 40 mm diameter. Determine the crack width when the section is subjected to a BM of 500 Kn m at a point on the side of the beam 250 mm below the neutral axis. [12M]
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**UNIT-II****DESIGN OF DEEP BEAMS**

1. Design a single span deep beam to suit the following data and also draw reinforcement details in deep beam. [12M]  
Effective span of the beam=6m  
Overall depth of the beam=6m  
Width of support =0.6m  
Width of beam=0.4 m  
Total load on beam including self weight= 400KN/m  
Concrete= M20 Grade  
Steel =Fe415 HYSD bars
  2. Design a 3span continuous deep beam carrying a characteristic load of 210KN/m inclusive of its self weight for the beam, clear span is 4.5 m width of support 250 mm. thickness of the beam 230 mm and Overall depth of 2.7 m. the materials are M20 Grade concrete and and HYSD reinforced of grade Fe415. . [12M]
  3. Simply supported beam of 250 mm wide and 1500 mm overall depth & 2300 mm clear span is simply supported on 200 mm wide support on either side it carries UDL of 200KN/m inclusive of its self weight. Design the beam using M20 concrete and Fe415 Grade. . [12M]
  4. Explain the procedure for continuous deep beam and draw the reinforcement details.[12M]
  5. Design a continuous deep beam having more than 3 spans and loaded a UDL of 180KN/m inclusive of self weight for the beam the clear span 5 mts.width of supports 300 mm beam thickness 250 mm. Overall thickness of beam is 3.5 mts. The material used are M20 HYSD bars of 415. [12M]
  6. A simply supported deep beam 200 mm wide x 1800 mm overall depth and 2750 mm clear span is simply supported on 250 mm wide supports on either sides. It carries a characteristic UDL of 260KN/m inclusive of its self weight. Design and details the beam. The materials are M20 Grade concrete and HYSD reinforced of grade Fe415. [12M]
  7. The reinforced concrete beam girder is continuous over spans of 8 m apart from c/c. It is 4.4 m deep and 330 mm thick and supports of column 900 mm width. If the girder supports a UDL of 210KN/m inclusive its own weight. Design Deep beam using M20 Grade concrete and Fe415 Grade steel. . [12M]
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8. Simply supported beam of 250 mm wide and 1500 mm overall depth & 2300 mm clear span is simply supported on 200 mm wide support on either side it carries UDL of 200kN/m inclusive of its self weight. Design the beam using M20 concrete and Fe415 Grade. . [12M]
9. A reinforced concrete deep girder is continuous over span of 10 m apart from centre to centre. It is 4.6 m deep, 300 mm thick and the supports are columns 900 mm in width. If the girder supports a UDL of 180 kN/m, design the reinforcement required if M20 concrete and Fe415 steel is used. [12M]
10. Design a simply supported deep beam to the following data: [12M]
- Clear span = 4.20 m
  - Bearing at each end = 450 mm
  - Overall depth = 3500 mm
  - Width of beam = 250 mm
  - Super imposed load = 250 kN/m
  - Use M20 concrete and Fe415 steel.
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**UNIT-III**  
**DESIGN OF FLAT SLAB**

1. A flat plate  $7 \times 6$  m panel on  $500 \times 500$  mm column has a slab thickness of 180 mm, designed for a load of  $9.3 \text{ kN/m}^2$ . Check for the safety of slab in shear and also determine the necessary stirrups for reinforcing the slab. Adopt M25 and Fe 415. . [12M]
2. A flat plate with  $7.5 \times 7.5$  m panels on  $500 \times 500$  mm columns has a slab thickness of 180 mm, designed for a total load of  $9.0 \text{ kN/m}^2$ . Check the safety of slab in shear and also find the stirrups for reinforcing in the slab. Use M25 and Fe415. . [12M]
3. Design an interior panel o a flat slab of size 5 m X 5 m without providing drop and column head. Size of column is  $500 \times 500$  mm and live load on the panel is  $4 \text{ kN/m}^2$ . Take floor finishing load as  $1 \text{ kN/m}^2$ . Use M20 Concrete and Fe 415 steel. [12M]
4. Design the interior panel of a flat-slab floor system for a ware house 24m divided into panels of  $6 \text{ m} \times 6 \text{ m}$ . . [12M]  
Loading class =  $5 \text{ kN/m}^2$   
Materials : M20 grade concrete, Fe 415 HYSD bars  
Column size = 400 mm diameter
5. Design the exterior panel of a flat-slab floor system for a ware house 24m divided into panels of  $6 \text{ m} \times 6 \text{ m}$ . . [12M]  
Loading class =  $4 \text{ kN/m}^2$   
Materials : M25 grade concrete, Fe 415 HYSD bars  
Column size = 400 mm diameter
6. A flat plate with  $7.5 \times 7.5$  m panels on  $500 \times 500$  mm columns has a slab thickness of 180 mm, designed for a total load of  $8.0 \text{ kN/m}^2$ . Check the safety of slab in shear and also find the stirrups for reinforcing in the slab. Use M20 and Fe415 . [12M]
6. A flat plate with  $9 \times 9$  m panels on  $500 \times 500$  mm columns has a slab thickness of 180 mm, designed for a total load of  $9.0 \text{ kN/m}^2$ . Check the safety of slab in shear and also find the stirrups for reinforcing in the slab. Use M25 and Fe415. . [12M]
7. Design an interior panel o a flat slab of size 8m X 8 m without providing drop and column head. Size of column is  $500 \times 500$  mm and live load on the panel is  $4 \text{ kN/m}^2$ . Take floor finishing load as  $1 \text{ kN/m}^2$ . Use M20 Concrete and Fe 415 steel. [12M]
8. Design the interior panel of a flat-slab floor system for a ware house 24m divided into panels of  $8 \text{ m} \times 8 \text{ m}$ . . [12M]  
Loading class =  $5 \text{ kN/m}^2$   
Materials : M20 grade concrete, Fe 415 HYSD bars

Column size = 400 mm diameter

9. A flat plate  $9 \times 6$  m panel on  $500 \times 500$  mm column has a slab thickness of 180 mm, designed for a load of  $9.3 \text{ kN/m}^2$ . Check for the safety of slab in shear and also determine the necessary stirrups for reinforcing the slab. Adopt M25 and Fe 415. . [12M]
10. A flat plate with  $8 \times 8$  m panels on  $500 \times 500$  mm columns has a slab thickness of 180 mm, designed for a total load of  $9.0 \text{ kN/m}^2$ . Check the safety of slab in shear and also find the stirrups for reinforcing in the slab. Use M25 and Fe415. . [12M]
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## UNIT-IV

## DESIGN OF RIBBED (VOIDED) SLABS

1. A simply supported one way ribbed slab of 5 m span is to be used for  $3 \text{ KN/m}^3$  live load. Design the slab using M20 grade concrete and HYSD bars of grade Fe 415. .[12M]
  2. Design a continuous ribbed slab with 3 equal spans of 5.8 m. the ribs supports on the beam with over span is 250 mm x 600 mm. take live load on the slabs is  $3 \text{ KN/m}^2$  use M20 Grade concrete and Fe415 steel. [12M]
  3. Explain the Analysis and Design procedure for ribbed Slabs .[12M]
  4. A simply supported one way ribbed slab of 6 m span is to be used for  $5 \text{ KN/m}^3$  live load. Design the slab using M20 grade concrete and HYSD bars of grade Fe 415. . [12M]
  5. Write short notes on: . [12M]
    - (a)Shear effect in two-way slab with beams.
    - (b)Flat slabs with opening.
    - (c)ACI guidelines for shear calculations.
    - (d)Strengthening of columns for shear and torsion
  6. Short note on: [12M]
    - (a)Shear due to unbalanced moments.
    - (b)Effect of opening in flat slab.
    - (c)Strengthening of column areas for moment transfer.
    - (d)Shear design guidelines as per ACI code.
  7. A simply supported one way ribbed slab of 5m span is to be used for  $3 \text{ kn/m}^2$  live load design the slab using M20 grade concrete and Fe 415 HYSD bars. . [12M]

Ribs are spaced at 450mm c/c.  
The thickness of topping as 60 mm.  
Width of rib as 120mm.  
Over all depth is 300mm.
  8. Design a continuous ribbed slab with 3 equal spans at 5.8m the ribs support on the beam with over span is 250mm x 600mm. take live load on the slab is  $3\text{Kn/m}^2$  use M20 grade concrete and Fe415 steel. . [12M]

Ribs are spaced at 450mm c/c
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The thickness of topping as 60 mm

Width of rib as 120mm

Over all depth is 300mm

9. A simply supported one way ribbed slab of 6m span is to be used for  $5 \text{ kn/m}^2$  live load design the slab using M25 grade concrete and Fe 415 HYSD bars. .[12M]

Ribs are spaced at 450mm c/c.

The thickness of topping as 50 mm.

Width of rib as 120mm.

Over all depth is 250mm.

10. Design a continuous ribbed slab with 4 equal spans at 5.8m the ribs support on the beam with over span is 250mm x 600mm. take live load on the slab is  $4 \text{ Kn/m}^2$  use M25 grade concrete and Fe415 steel. . [12M]

Ribs are spaced at 450mm c/c

The thickness of topping as 60 mm

Width of rib as 120mm

Over all depth is 300mm



## UNIT-V

## DESIGN OF PLAIN CONCRETE WALLS AND SHEAR WALLS

1. A plain braced concrete wall of dimensions 8 m high, 6m long and 200 mm thick is restrained against rotation at its base and unrestrained at the ends. If it has to carry a factored total gravity load of 200 KN and a factored horizontal load of 8 KN at top. Check the safety of the wall. Assume M20 concrete and Fe 415 steel. . [12M]

2. Estimate the reinforcement of a shear wall of length 4.16 m and thickness 250 mm is subjected to the following forces  $f_{ck}=25 \text{ N/mm}^2$ ,  $f_y= 500 \text{ N/mm}^2$  .[12M]

S.No	Loading	Axial Load (KN)	Shear Force (KN)	Bending Moment (KN-m)
1	DL + LL	1950	600	20
2	Seismic Load	250	4800	70

3.Design a shear wall of length 5.0 m and thickness 250 mm subjected to the forces given below and the wall is a high wall with the following loadings. Use M25 and Fe415. .[12M]

S.No	Loading	Axial Load (KN)	Shear Force (KN)	Bending Moment (KN-m)
1	DL + LL	1950	500	20
2	Seismic Load	200	4500	80

4.Design a shear wall subjected to  $P_u=12000\text{kn}$  and  $M_u=11000\text{kn.m}$ . the materials used are M30 grade concrete and Fe 415 steel and thickness of wall is 200mm and length is 6m design the wall

- Using interaction chart . [12M]
- Using elastic stress distribution design end portion of height 600mm
- Assume end zone to resist moment and 500mm X 500mm column at end zone.

5. Design a shear wall subjected to  $P_u=10000\text{kn}$  and  $M_u=8000\text{kn.m}$ . the materials used are M30 grade concrete and Fe 415 steel and thickness of wall is 150mm and length is 5m design the wall

- Using interaction chart

- Using elastic stress distribution design end portion of height 600mm
  - Assume end zone to resist moment and 500mm X 500mm column at end zone.
6. Design a shear wall subjected to  $P_u=15000$  kn and  $M_u=11000$  kn.m. the materials used are M25 grade concrete and Fe 415 steel and thickness of wall is 200mm and length is 6m design the wall. [12M]
- Using interaction chart
  - Using elastic stress distribution design end portion of height 600mm
  - Assume end zone to resist moment and 500mm X 500mm column at end zone.
7. Briefly explain the classifications of shear wall with neat sketch? .[12M]
8. Explain the design procedure to design the shear wall? .[12M]
9. A plain braced concrete wall of dimensions 10 m high, 6m long and 200 mm thick is restrained against rotation at its base and unrestrained at the ends. If it has to carry a factored total gravity load of 250 KN and a factored horizontal load of 10 KN at top. Check the safety of the wall. Assume M25 concrete and Fe 415 steel. .[12M]
10. Design a shear wall of length 5.0 m and thickness 250 mm subjected to the forces given below and the wall is a high wall with the following loadings. Use M25 and Fe415. . [12M]

S.No	Loading	Axial Load (KN)	Shear Force (KN)	Bending Moment (KN-m)
1	DL + LL	1950	500	20
2	Seismic Load	200	4500	80

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